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222 NORTH STREET, RAYMOND HINGHAM, MA. **DESIGN ASSOCIATES** 02043

RRC **%** RC ENGINEERING SOUTH STREET, WRENT WRENTHAM, MA. 02093

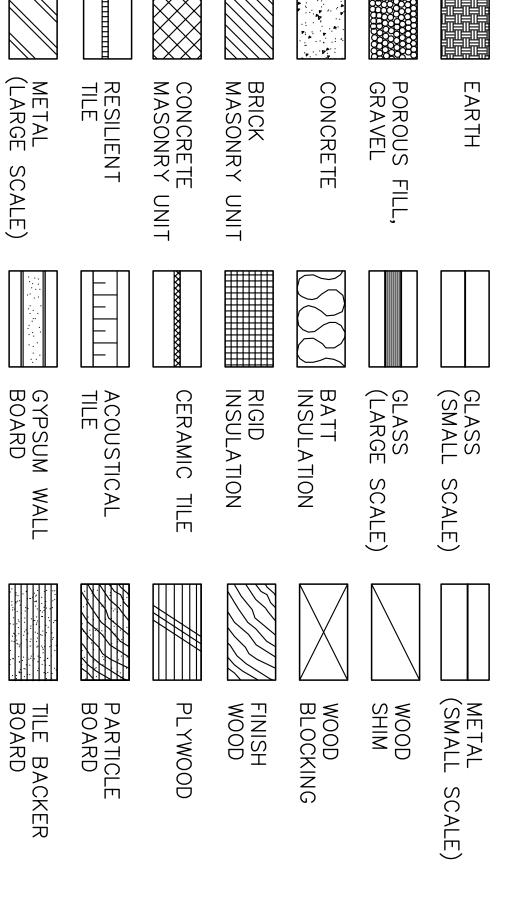
WB&A WOZNY/B, 1090 WASHINGTON ST. WOZNY/BARBAR & INGTON ST. HANOVER, **ASSOCIATES** MA 02339 NC.

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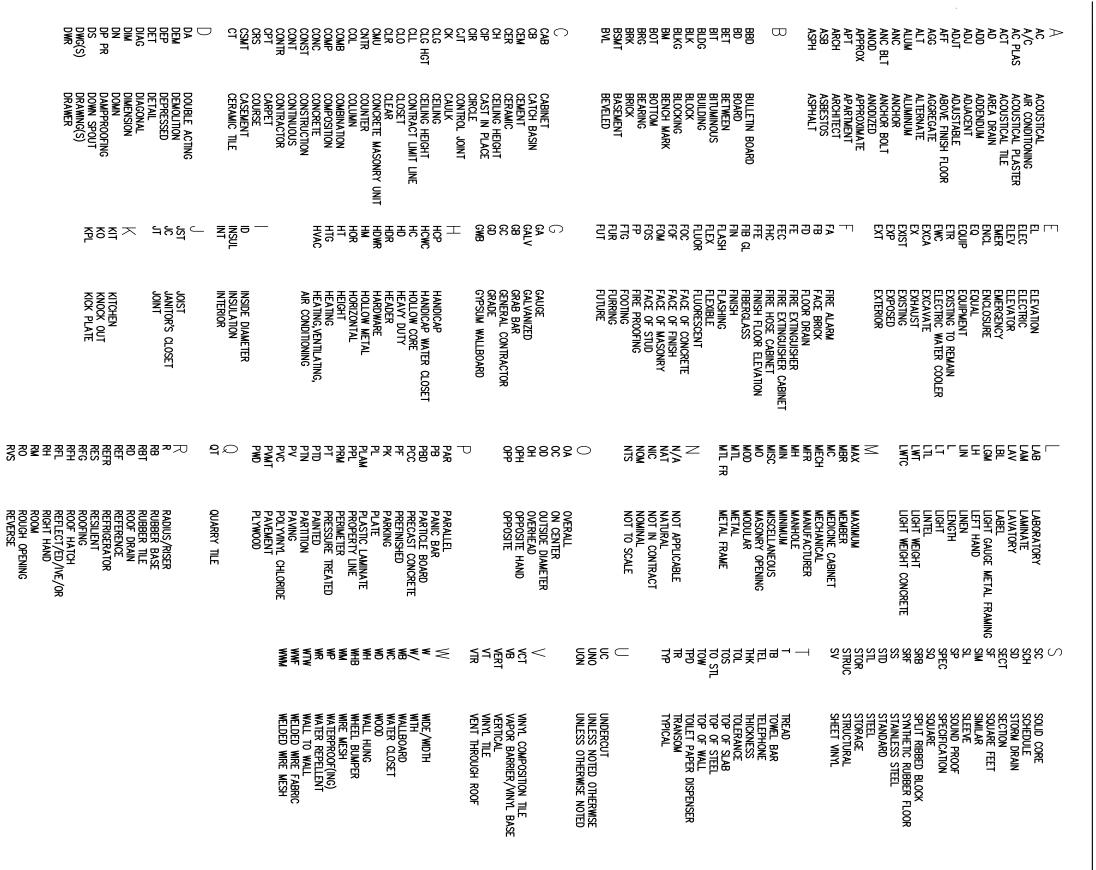
STRUCTURAL ENGINEER

PLUMBING, MECHANICAL H ECTRICAL ENGINEER 80

MATERIAL **SYMBOLS**



BBREVIATIONS



OF DRAWINGS

architecture

<u>ARCHITECTURAL</u>

CS1 C1 A1 A2

COVER SITE PLAN FLOOR PLAN SECTIONS / E V / REFLECTED CEILING PLAN / ROOF PLAN

Raymond Design

Associates, Inc.

STRUC TURAL

\$101 \$102 \$200 \$201 FOUNDATION PLAN & SECTIONS
CEILING / ROOF FRAMING PLANS
GENERAL NOTES & QUALITY ASSURANCE
TYPICAL DETAILS

PLAN

Hingham, MA 02043

222 North Street

PLUMBING

PLUMBING PLAN, LEGEND ૹ

M1.1 M2.1

IANICAL

MECHANICAL MECHANICAL SCHEDULES, FLOOR PLAN

TRICAL

E0.0 E1.0 E2.0

ELECTRICAL LEGEND & GENERAL NOTES ELECTRICAL LIGHTING & POWER PLANS ELECTRICAL RISERS SCHEDULES, & DETAILS

DETAILS, LEGENDS & NOTES

NEWTON SOUTH HIGH SCHOOL FIELD FACILITY BUILDING 140 BRANDEIS ROAD **NEWTON CENTER MASSACHUSETTS 02459**

COVER SHEET

Revisions:

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DOOR NUMBER

WINDOW TYPE

LOUVER

TYPE

GRAPHIC

SYMBOLS

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COLUMN GRID

\(\frac{\frac{1}{25}}{25}\)

WALL

SECTION

DETAIL

ELEVATION MARKER

INTERIOR ELEVATION

ROOM NUMBER

REVISION MARKER

BUILDING

SECTION

WALL

TYPE MARKER

SHEET BID SET

Drawn By: Checked By: Approved By:

SRL

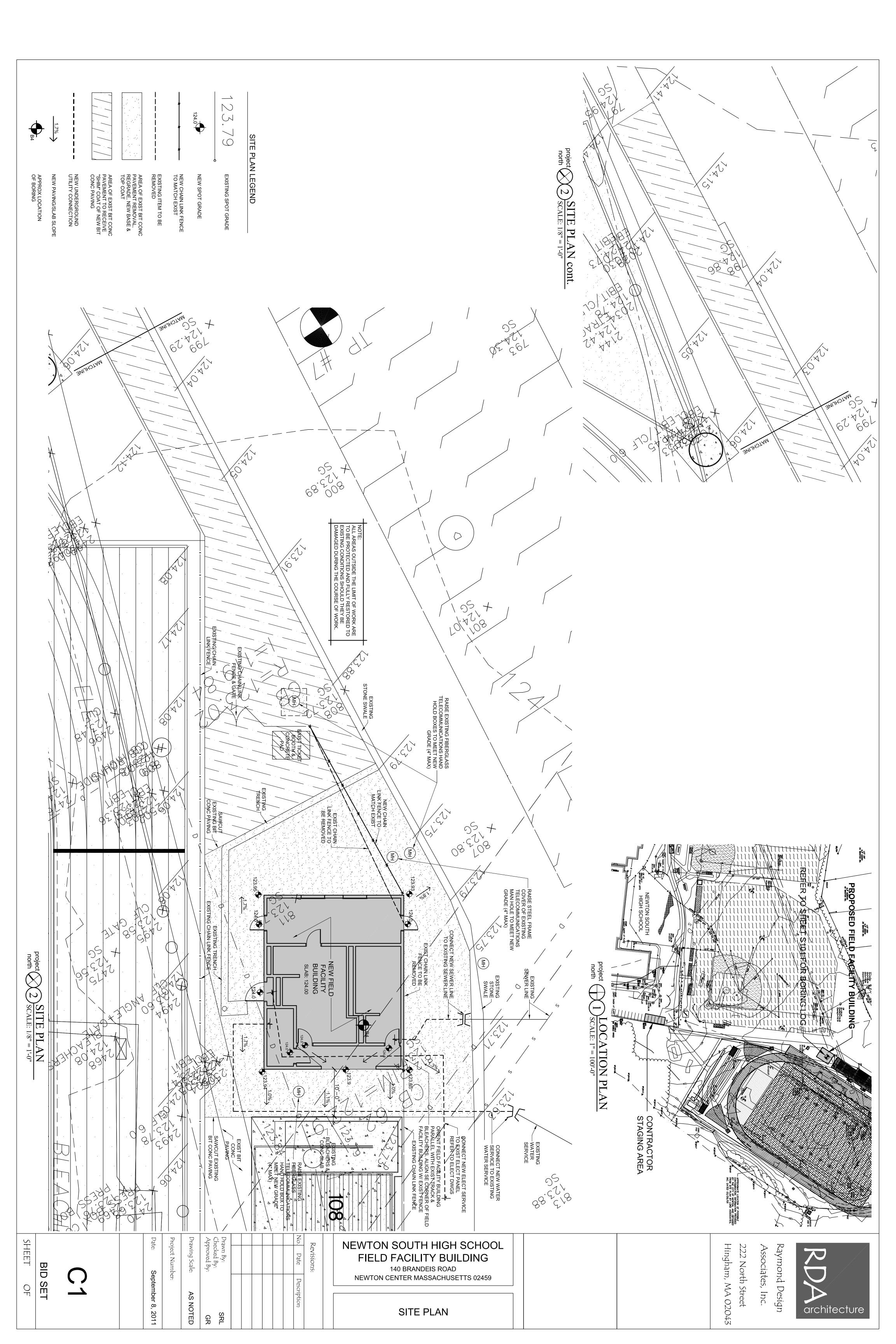
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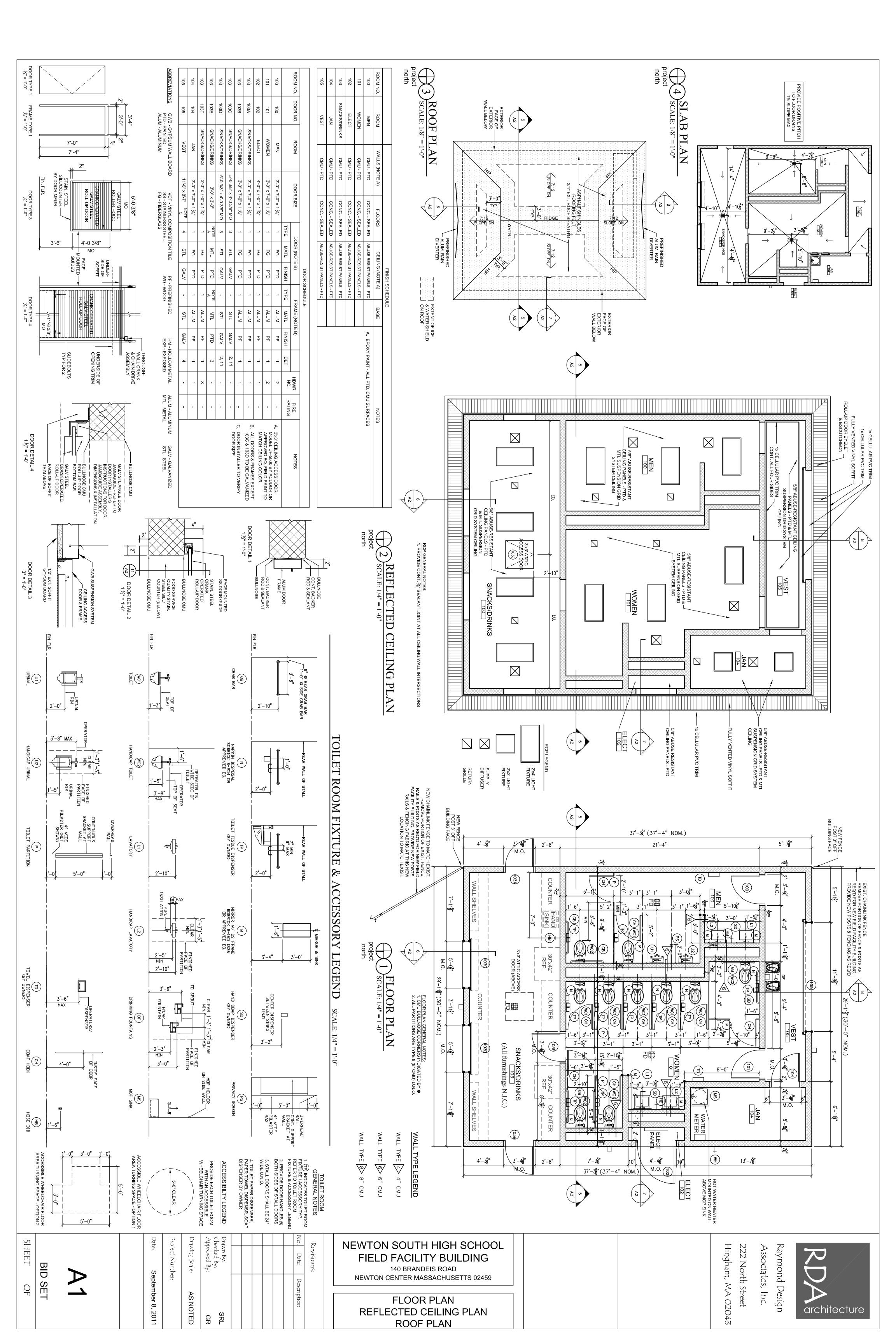
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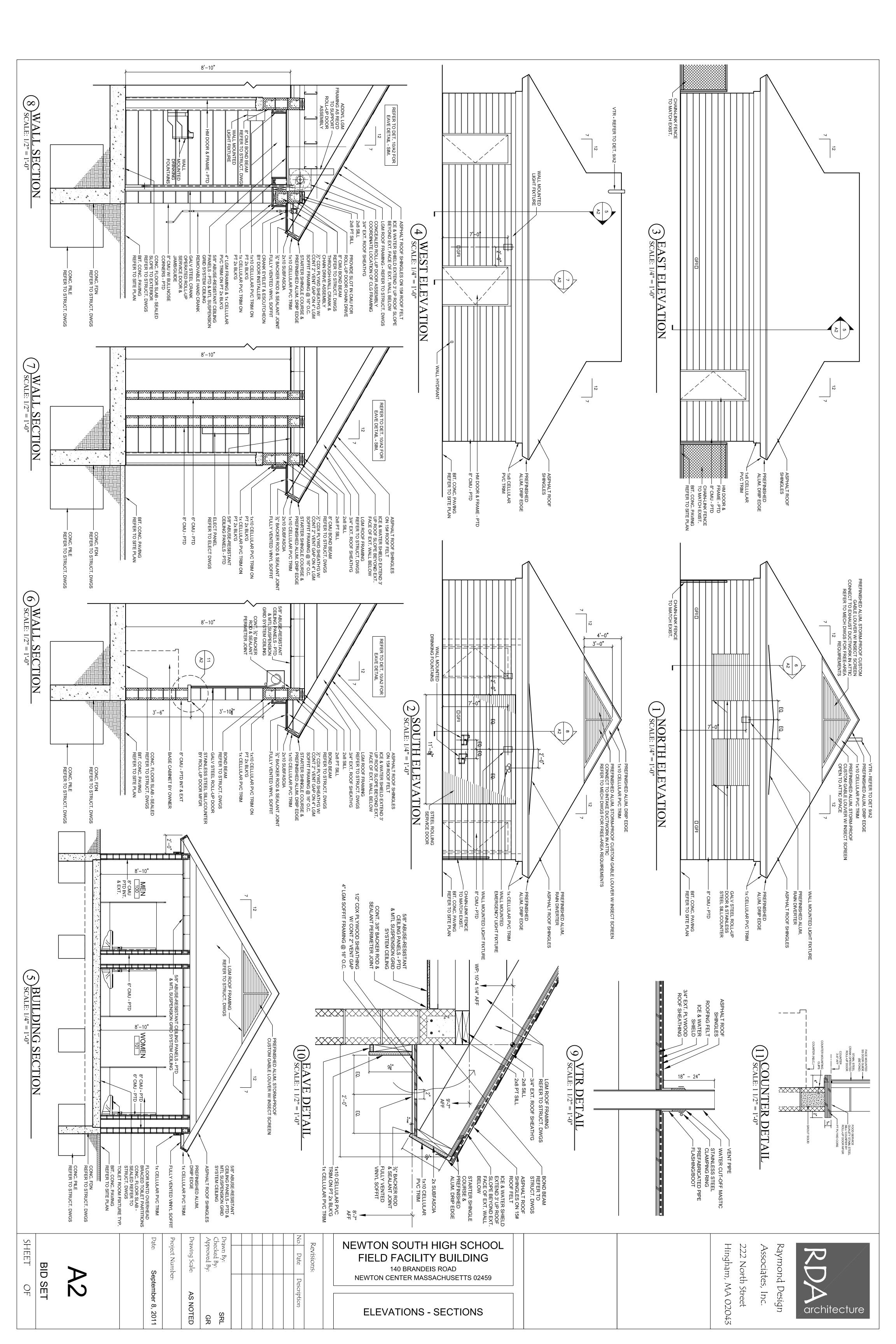
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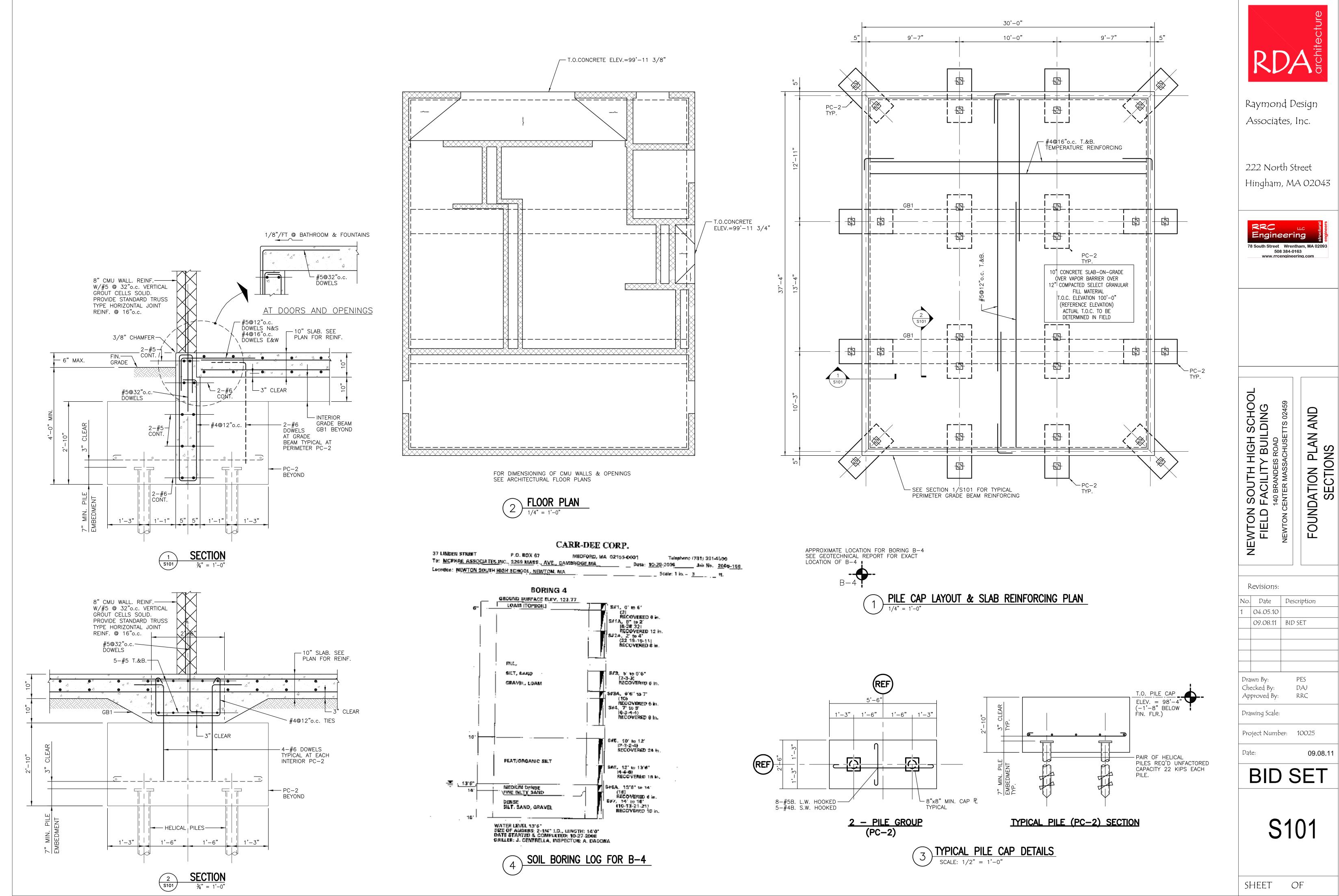
Date:

September 8, 2011









Raymond Design

222 North Street Hingham, MA 02043



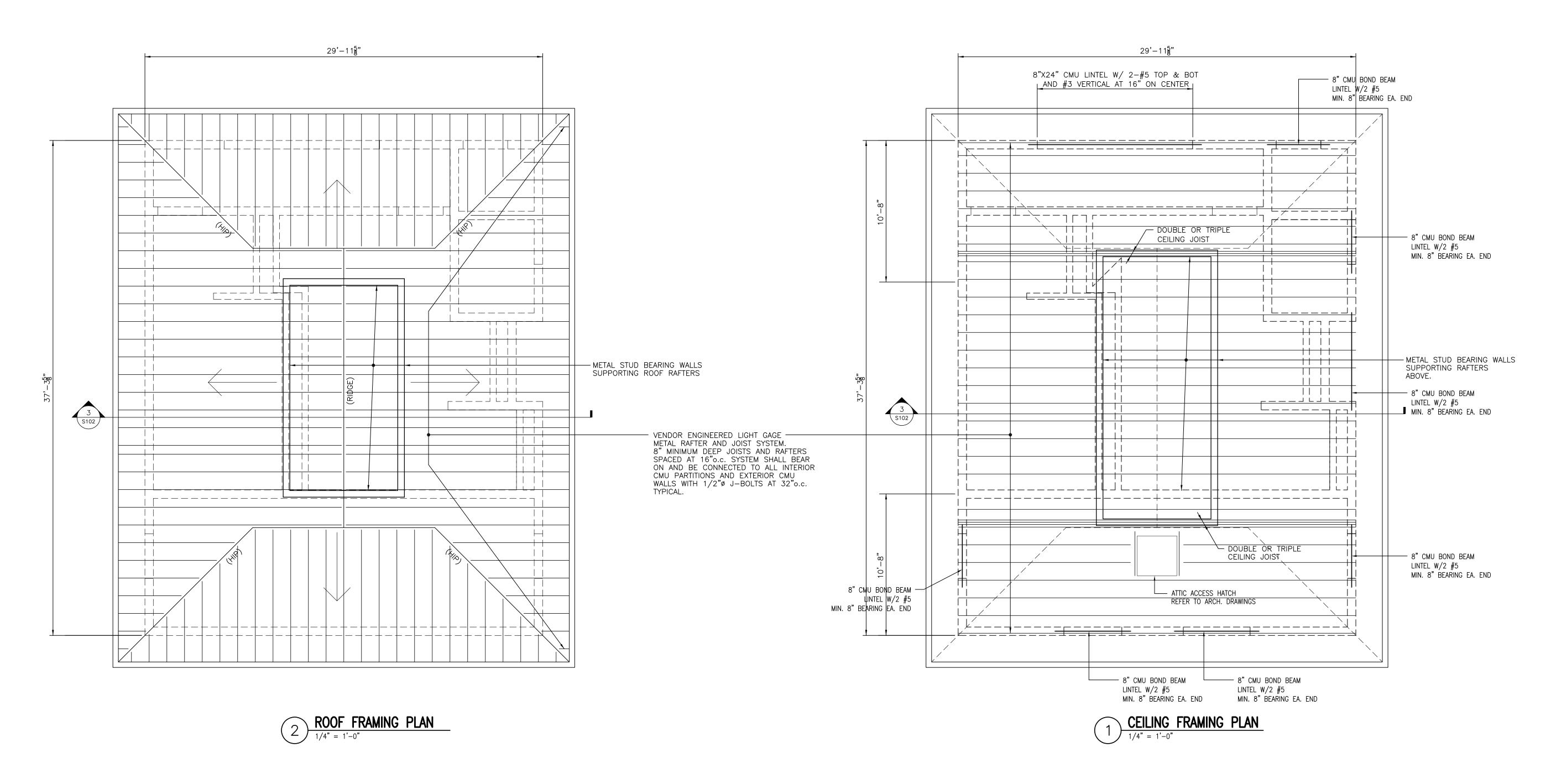
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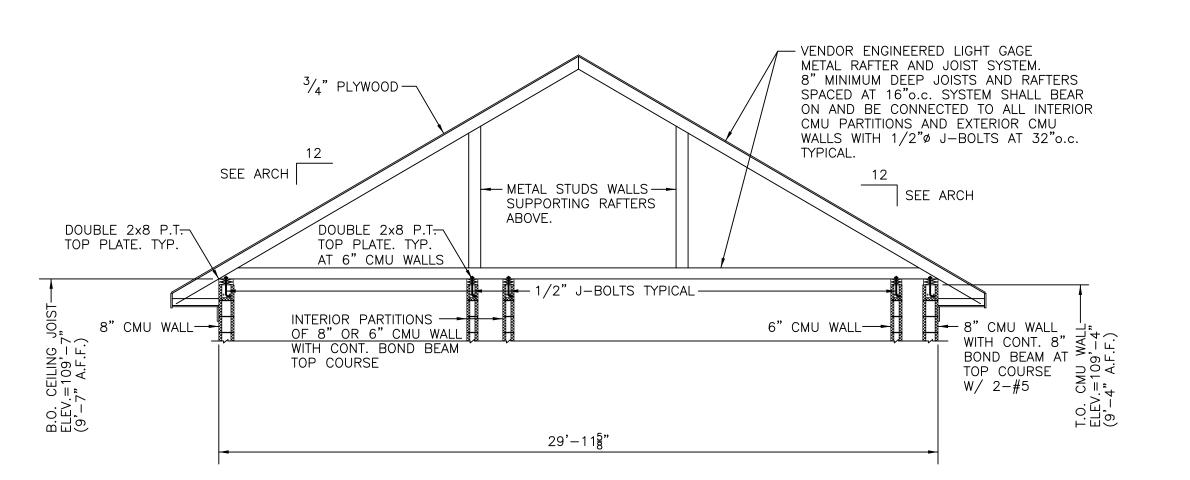
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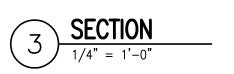
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Raymond Design Associates, Inc.

222 North Street Hingham, MA 02043



HIGH SCHOOL

Y BUILDING

SIS ROAD

SACHUSETTS 02459 NEWTON FIELD I

FRAMING PLAN FRAMING PLAN

CEILING & ROOF

Revisions: No. Date Description 04.05.10 09.08.11 | BID SET Drawn By: PES DAJ Checked By: RRC Approved By:

Drawing Scale:

Project Number: 10025

Date: 09.08.11

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OF

GENERAL NOTES

- 1. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
- 2. THE STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE SEVENTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE.
- 3. THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AND BRACING REQUIRED TO ERECT AND HOLD NEW STRUCTURES IN PROPER ALIGNMENT UNTIL PERMANENT SUPPORTS AND LATERAL BRACING ARE IN PLACE.
- 4. WHERE CONSTRUCTION OCCURS WITHIN OR ADJACENT TO EXISTING CONSTRUCTION, THE CONTRACTOR SHALL FIELD MEASURE THE EXISTING BUILDING DIMENSIONS AND COMPONENTS AND COORDINATE CONSTRUCTION DETAILS WITH THE ACTUAL DIMENSIONS.
- 5. ALL CONSTRUCTION IS NEW UNLESS SPECIFICALLY NOTED AS EXISTING (E).
- 6. THE STRUCTURE WAS DESIGNED FOR THE FOLLOWING LOADS:

FLOOR LIVE LOADS

PUBLIC AND SERVING AREAS 40 PSF ROOF SNOW LOADS

GROUND SNOW, Pq = 55 PSFFLAT ROOF SNOW, Pf = 38.5 PSF

WIND LOADS BASIC WIND SPEED, V = 105 MPH WIND IMPORTANT FACTOR, Iw = 1.0WIND EXPOSURE C

EARTHQUAKE DESIGN DATA

SEISMIC IMPORTANCE FACTOR 1.0 SEISMIC USE GROUP I MAPPED SPECTRAL RESPONSE ACCELERATIONS Ss = 0.27 AND S1 = 0.068

SPECTRAL RESPONSE COEFFICIENTS SDS = 0.288 AND SD1 = 0.109SITE CLASS D SEISMIC DESIGN CATEGORY B

BASE STRUCTURAL SYSTEM - CONCRETE ONE WAY SLAB ON GRADE BEAMS AND PILE CAPS BASE SEISMIC FORCE RESISTING SYSTEM - SPECIAL REINFORCED MASONRY SHEAR WALLS DESIGN BASE SHEAR V = 16,500 lbs SEISMIC RESPONSE COEFFICIENT, Cs = 0.058 RESPONSE MODIFICATION FACTOR, R = 3.5

ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE 7. MECHANICAL UNIT WEIGHTS AND LOCATIONS SHOWN ON THE PLANS ARE APPROXIMATE. THE

FOUNDATION NOTES

THE ARCHITECT.

1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL REPORT ENTITLED "PRELIMINARY GEOTECHNICAL ENGINEERING REPORT, NEWTON SOUTH HIGH SCHOOL ATHLETIC CAMPUS, PROJECT #4891, for GALE ASSOCIATES", PREPARED BY McPHAIL ASSOCIATES, INC., DATED NOVEMBER 18, 2008.

CONTRACTOR SHALL VERIFY LOCATIONS AND WEIGHTS SHOWN AND REPORT DISCREPANCIES TO

- 2. DESIGN OF FOUNDATIONS IS BASED ON AN ASSUMED HELICAL PILE CAPACITY OF 22 KIPS EACH. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR RETAINING THE SERVICES OF A GEOTECHNICAL ENGINEER REGISTERED IN THE COMMONWEALTH OF MASSACHUSETTS TO DESIGN A HELICAL PILE SYSTEM CAPABLE OF SUPPORTING THE ASSUMED PILE LOADS. A FINAL GEOTECHNICAL REPORT IS TO BE PROVIDED BY THE GEOTECHNICAL ENGINEER.
- 3. EXTERIOR GRADE BEAMS SHALL PENETRATE A MINIMUM OF 4'-0" BELOW EXTERIOR FINISHED
- 4. SUBGRADE BENEATH FOUNDATIONS AND CONCRETE SLABS SHALL BE COMPACTED TO A MINIMUM DRY DENSITY OF 95% AS DETERMINED BY ASTM D1557.
- 5. FOUNDATIONS SHALL BE TEMORARILY BRACED OR HAVE PERMANENT BRACING IN PLACE PRIOR TO BACKFILLING.
- 6. BACKFILL SHALL BE PLACED SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION TO THE GRADES INDICATED. WHERE THE EXTERIOR GRADE IS MORE THAN TWO FEET BELOW THE FLOOR SLAB ELEVATION, FOUNDATION WALLS SHALL BE BRACED UNTIL THE FLOOR SLAB HAS BEEN IN PLACE FOR AT LEAST 14 DAYS.
- 7. PILE CAPS. GRADE BEAMS AND STRUCTURAL SLABS SHALL NOT BE PLACED ON SUBGRADES CONTAINING STANDING WATER, SNOW, FROST, OR ICE.
- 8. THE SOIL SUBGRADE SHALL BE PROTECTED FROM FREEZING WITH INSULATING BLANKETS, TEMPORARY HEAT, OR OTHER MEANS. CONCRETE NOTES
- 1. ALL CONCRETE WORK SHALL CONFORM TO THE LATEST EDITION OF THE AMERICAN CONCRETE INSTITUTE PUBLICATIONS ACI 301, ACI 315, AND ACI 318.
- 2. ALL CONCRETE SHALL BE NORMAL WEIGHT AND HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTHS:

PILE CAPS AND PERIMETER GRADE BEAMS f'c = 3.000 PSI CEMENTITIOUS MATERIALS CONTENT 517 #/CY

WATER CEMENT RATIO 0.5 4" PLUS/MINUS 1"

STRUCTURAL SLABS AND INTERIOR GRADE BEAMS f'c = 4,000 PSI CEMENTITIOUS MATERIALS CONTENT 611 #/CY WATER CEMENT RATIO 0.45

4" PLUS/MINUS 1" NO AIR-ENTRAINING ADMIXTURE EXTERIOR SLABS f'c = 4,000 PSI

CEMENTITIOUS MATERIALS CONTENT 611 #/CY WATER CEMENT RATIO SLUMP 4" PLUS/MINUS 1" AIR CONTENT 6% PLUS/MINUS 1%

3. REINFORCING STEEL SHALL BE AS FOLLOWS: REINFORCING BARS - ASTM A615 GRADE 60 WELDED WIRE FABRIC - ASTM A185

- 4. FLY ASH ADDITIVES SHALL NOT BE USED FOR CONCRETE SLABS OR ARCHITECTURALLY EXPOSED
- 5. ALL REINFORCING STEEL AND EMBEDDED ITEMS SHALL BE ACCURATELY PLACED IN THE POSITIONS SHOWN AND ADEQUATELY TIED AND SUPPORTED BEFORE CONCRETE IS PLACED TO PREVENT DISPLACEMENT BEYOND PERMITTED TOLERANCES.
- 6. DOWELS SHALL BE ACCURATELY PLACED AND SECURELY TIED IN POSITION BEFORE CONCRETE IS PLACED. DOWELS SHALL NOT BE INSTALLED INTO WET CONCRETE.

7. MINIMUM COVER TO REINFORCEMENT SHALL BE:

CAST AGAINST SOIL FORMED SURFACES EXPOSED TO GROUND 2" TOP OF EXTERIOR SLABS 2' INTERIOR PIERS, PILASTERS 1 1/2" INTERIOR WALL SURFACES TOPS OF INTERIOR SLABS

- 8. ALL CONTINUOUS REINFORCEMENT SHALL HAVE A MINIMUM SPLICE AS REQUIRED FOR A CLASS B SPLICE, PER ACI 318, SECTION 12.15, UNLESS OTHERWISE NOTED.
- 9. CURE CONCRETE SLABS BY COVERING WITH A MOISTURE RETAINING COVER AND KEEPING THE SURFACE CONTINUALLY WET FOR AT LEAST 7 DAYS.

- 10. CURE FORMED SURFACES BY MOIST CURING WHILE FORMS REMAIN IN PLACE. AFTER REMOVAL OF FORMS, APPLY A LIQUID MEMBRANE-FORMING CURING COMPOUND COMPLYING WITH ASTM C309. TYPE I. PER MANUFACTURER'S RECOMMENDATIONS.
- 11. VAPOR RETARDER SHALL BE PLACED BENEATH A 4" THICK LAYER OF GRANULAR FILL. VAPOR RETARDER SHALL BE GRIFFOLYN VAPORGUARD BY REEF INDUSTIES, STEGO WRAP (15 MILS) VAPOR BARRIER BY STEGO INDUSTRIES LLC, OR PREMOULDED MEMBRANE WITH PLASTMATIC CORE BY W. R. MEADOWS. MINIMUM VAPOR RETARDER THICKNESS SHALL BE 10 MILS, AND SHALL CONFORM TO ASTM E1745, CLASS A OR B.
- 12. UNLESS NOTED OTHERWISE, ANCHORING ADHESIVE FOR REBAR DOWELS SHALL BE HILTI HIT RE 500 EPOXY ANCHORING SYSTEM.
- 13. SUBMIT THE FOLLOWING TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION OR CONSTRUCTION:
- a. CONCRETE MIX DESIGN AND TEST REPORTS FOR THE PROPSED CONCRETE MIXES b. PRODUCT DATA FOR MATERIALS, ADMIXTURES, AND ACCESSORIES c. REINFORCING STEEL SHOP DRAWINGS

CONCRETE MASONRY NOTES

- 1. CONCRETE MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTUTE (ACI) "BUILDING CODE REQUREMENTS FOR CONCRETE MASONRY STRUCTURES" ACI 530-95 / ASCE 5-95.
- 2. CONCRETE MASONRY CONSTRUCTION SHALL CONFORM TO THE "SPECIFICATIONS FOR CONCRETE MASONRY STRUCTURES" ACI 530.1-95 / ASCE 6-95.
- 3. THE COMPRESSIVE STRENGTH OF MASONRY, I'm, EXPRESSED AS FORCE PER UNIT OF NET CROSS-SECTIONAL AREA, SHALL BE 1,500 PSI AT 28 DAYS.
- 4. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, TYPE 2 AND BE MADE WITH NORMAL WEIGHT AGGREGATE. COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS SHALL COMPLY WITH THE FOLLOWING

1,900 PSI FOR f'm = 1,500 PSI

- 5. REINFORCING STEEL SHALL COMPLY WITH ASTM A 615, GRADE 60. SHOP-FABRICATE REINFORCING BARS WHICH ARE SHOWN TO BE BENT OR HOOKED.
- 6. HORIZONTAL JOINT REINFORCING SHALL CONFORM TO ASTM A951 AND BE HOT-DIPPED GALVANIZED.
- 7. GROUT SHALL COMPLY WITH ASTM C 476 AND SHALL BE PROPORTIONED TO OBTAIN THE FOLLOWING 28-DAY COMPRESSIVE STRENGTHS:

2,000 PSI FOR f'm = 1,500 PSI

- 8. MORTAR SHALL COMPLY WITH ASTM C 270, TYPE S OR M. AGGREGATE FOR MORTAR SHALL COMPLY WITH ASTM C 144. USE TYPE M MORTAR BELOW GRADE AND TYPE S MORTAR ABOVE
- 9. ALL MASONRY CORES CONTAINING VERTICAL REINFORCING SHALL BE GROUTED SOLID.
- 10. LAP REINFORCING BARS 48 X BAR DIAMETER AT ALL SPLICES.
- 11. PROVIDE REBAR DOWELS FROM THE FOUNDATION OF THE SAME SIZE AND SPACING AS VERTICAL REINFORCING. DOWELS SHALL HAVE STANDARD ACI HOOKS.
- 12. ALL CMU CORES BELOW FINISHED GRADE SHALL BE GROUTED SOLID.
- 13. PROVIDE BAR POSITIONERS FOR ALL VERTICAL REINFORCING AT 200 X BAR DIAMETER
- 14. SUBMIT THE FOLLOWING TO THE ARCHITECT FOR REVIEW PRIOR TO CONSTRUCTION:

LETTER OF COMPLIANCE OF MASONRY UNITS WITH ASTM C90 GROUT MIX DESIGN AND TEST REPORTS FOR THE PROPOSED MIX REINFORCING STEEL SHOP DRAWINGS

STRUCTURAL COLD-FORMED METAL FRAMING

- 1. ALL STUDS, JOISTS, HEADERS, TRACK AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT MEETS THE REQUIREMENTS OF THE LATEST EDITION OF AISI SPECIFICATIONS AND STANDARDS. ALL MATERIALS SHALL BE FORMED FROM STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A653 AND BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A653-G60.
- MINIMUM YIELD STRENGTH SHALL BE 33,000 PSI FOR ALL TRACK, 20 GA. AND 18 GA. MEMBERS, AND 50,000 PSI FOR ALL 16 GA., 14 GA., AND 12 GA. MEMBERS.
- 3. PROVIDE COLD FORMED CHANNEL BRIDGING FOR LATERAL BRACING OF COMPRESSION MEMBERS AT A MINIMUM SPACING OF 48" O.C.
- 4. PROVIDE WEB STIFFENERS AT ALL JOIST BEARING POINT LOCATIONS. JOIST WEBS SHALL BE ALIGNED WITH STUDS IN BEARING WALLS.
- AT BOTH TOP AND BOTTOM OF WALL.

5. STUDS SHALL BE INSTALLED WITH FULL BEARING AGAINST THE INSIDE OF RUNNER TRACK

- 6. ANCHOR RUNNER TRACK TO CONCRETE SLAB WITH 0.177 DIAMETER POWDER DRIVEN FASTENERS SPACED AT A MINIMUM OF 16" O.C., UNLESS OTHERWISE INDICATED ON DRAWINGS.
- 7. ALL FRAMING, INCLUDING STUDS, JOISTS, HEADERS, SILLS, TRACKS, ACCESSORIES AND CONNECTIONS SHALL BE DESIGNED BY THE CONTRACTOR IN ACCORDANCE WITH THE LATEST EDITION OF AISI "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS." DESIGNS SHALL BE PREPARED BY A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
- 8. SUBMIT THE FOLLOWIING TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION OR CONSTRUCTION:

CALCULATIONS, STATEMENT OF DESIGN CRITERIA, ENGINEERING ANALYSIS, INDICATION STRESS AND DEFLECTION FOR EACH FRAMING APPLICATION, SELECTION OF FRAMING COMPONENTS AND ACCESSORIES, VERIFICATION OF ATTACHMENT TO STRUCTURE AND/OR ADJACENT FRAMING COMPONENTS.

SHOP DRAWINGS INCLUDING PLANS, SECTIONS, ELEVATIONS, DETAILS, AND CONNECTION REQUIREMENTS.

SUBMITTALS SHALL BE STAMPED AND SEALED BY A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.

- 9. THE GOVERNING BUILDING CODE SHALL BE REFERRED TO FOR DETERMINING DEAD LOADS AND LIVE LOADS. REFER TO THE CONSTRUCTION DOCUMENTS FOR ANY ADDITIONAL DESIGN LOADS. LOADING CONDITIONS SHALL BE CONSIDERED AS REQUIRED BY THE GOVERNING BUILDING CODE.
- 10. ALL MEMBERS SHALL BE DESIGNED FOR THE FOLLOWING DEFLECTION LIMITS:

ROOF LOAD, SNOW LOAD TOTAL DEAD PLUS LIVE LOAD

L/240

L/360

WOOD FRAMING

1. LUMBER SHALL BE SPRUCE-PINE-FIR NO. 1 / NO. 2 OR BETTER WITH THE FOLLOWING MINIMUM

ALLOWABLE BENDING STRESS Fb= 875 PSI ALLOWABLE SHEAR STRESS Fv= 70 PSI Fc= 1,100 PSI COMPRESSION MODULUS OF ELASTICITY E= 1,400,000 PSI

- 2. PRESSURE TREATED LUMBER SHALL BE USED WHERE INDICATED ON PLANS, IN ALL EXTERIOR APPLICATIONS, AND IN APPLICATIONS WHERE THE LUMBER IS IN CONTACT WITH GROUND. PRESSURE TREATED LUMBER SHALL BE SOUTHERN YELLOW PINE, NO. 1 GRADE. LUMBER SHALL BE TREATED WITH ALKALINE COPPER QUAT (ACQ) OR COPPER AZOLE (CBA) PRESERVATIVE.
- 3. FRAMING CONNECTORS AND FASTENERS FOR USE WITH PRESSURE TREATED LUMBER SHALL BE STAINLESS STEEL. ALTERNATIVELY, BATCH/POST HOT-DIPPED GALVANIZED FASTENERS (ASTM A153) AND FRAMING CONNECTORS (ASTM A123) SHALL BE USED HICK APA RATED SHEATHING, C-D
- 4. PLYWOOD ROOF SHEATHING SHALL BE 19/32? OR 5/8? EXPOSURE 1, 32/16 SPAN RATING. LONG DIMENSION SHALL BE PERPENDICULAR TO SUPPORTS. JOINTS SHALL BE STAGGERED. ? CC AROUND THE
- 5. FASTEN ROOF SHEATHING TO SUPPORTING MEMBERS WITH 10D NAILS AT 4 PERIMETER OF INDIVIDUAL SHEETS AND 12? CC WITHIN THE FIELD.

TESTING AND INSPECTIONS

- 1. THIS PROJECT IS SUBJECT TO CONSTRUCTION CONTROL PER THE MASSACHUSETTS STATE BUILDING CODE. AS PART OF CONSTRUCTION CONTROL REQUIREMENTS, THE ENGINEER HAS PREPARED AND SUBMITTED TO THE ARCHITECT A PROGRAM OF TESTS AND SPECIAL INSPECTIONS. THE CONTRACTOR SHALL COORDINATE THE ACTIVITIES OF THE SPECIAL INSPECTOR TO ENSURE THAT ALL TESTS NOTED IN THE PROGRAM ARE TAKEN.
- 2. THE OWNER WILL RETAIN THE SERVICES OF THE SPECIAL INSPECTOR TO PERFORM THE TESTS AND INSPECTIONS NOTED.
- 3. SPECIAL INSPECTONS WILL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE SIXTH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE. THE PROGRAM OF STRUCTURAL TESTS AND INSPECTIONS INCLUDES, BUT IS NOT LIMITED TO THE FOLLOWING:

EARTHWORK

STRUCTURAL FILL MATERIAL FILL LIFT THICKNESS IN-PLACE DENSITY

CONCRETE CONSTRUCTION CONCRETE MATERIALS TESTING LOCATION AND INSTALLATION DETAILS OF REINFORCING EVALUATION OF CONCRETE STRENGTH PROPER USE OF MIX PROPORTIONS CONCRETE PLACEMENT TECHNIQUES CURING TECHNIQUES AND CURING TEMPERATURES

MASONRY CONSTRUCTION

FULL TIME INSPECTION OF LOADBEARING MASONRY WALLS MASONRY STRENGTH PROPORTIONING AND MIXING OF MORTAR AND GROUT LOCATION AND INSTALLATION DETAILS OF REINFORCING PLACING OF MORTAR AND GROUT HOT AND COLD WEATHER PROTECTION

QUALITY ASSURANCE PROGRAM

- 1. THIS QUALITY ASSURANCE PROGRAM IS PREPARED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH 780 CMR 1701.4 OF THE 7TH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE.
- 2. ANY DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED. THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD.
- 3. JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.
- 4. MATERIALS AND ACTIVITIES TO BE INSPECTED ARE NOT TO INCLUDE THE CONTRACTOR'S EQUIPMENT OR METHODS USED TO ERECT OR INSTALL THE MATERIALS LISTED.
- 5. THE FOLLOWING CATEGORIES OF STRUCTURAL TESTS AND INSPECTIONS ARE INCLUDED IN THE PROGRAM FOR STRUCTURAL TESTS AND INSPECTIONS FOR THIS PROJECT. THE SPECIFIC TESTS AND INSPECTIONS REQUIRED FOR EACH CHECKED CATEGORY ARE LISTED IN DETAIL ON THIS

CAST-IN-PLACE CONCRETE MASONRY

6. THE FOLLOWING ITEMS OF CONSTRUCTION ARE SPECIFIED IN THE STRUCTURAL PLANS OR SPECIFICATIONS ON A PERFORMANCE BASIS. IN ACCORDANCE WITH 780 CMR 1705.3.4, THEIR STRUCTURAL DESIGNS WILL BE REVIEWED BY THE SER AND THEIR CONSTRUCTION IS INCLUDED IN THE PROGRAM FOR TESTS AND INSPECTIONS:

PILE DESIGN AND INSTALLATION LIGHT GAUGE METAL FRAMING

7. THE FOLLOWING ITEMS ARE EXCLUDED FROM THIS QUALITY ASSURANCE PROGRAM SINCE THEY ARE DESIGNED BY OTHER REPSONSIBLE DESIGN PROFESSIONALS NOT UNDER THE AEGIS OF THE STRUCTURAL ENGINEER OF RECORD AND THE STRUCTURAL ENGINEER OF RECORD WAS NOT RETAINED TO PROVIDE PERFORMANCE SPECIFICATIONS FOR THEIR DESIGN. THESE OTHER RESPONSIBLE DESIGN PROFESSIONALS MUST BE ASSIGNED BY THE OWNER OR ARCHITECT, AS APPLICABLE, TO PREPARE A SEPARATE QUALITY ASSURANCE PROGRAM FOR THEIR RESPECTIVE DESIGNS.

IN-SITU BEARING STRATA NON-STRUCTURAL ASSEMBLIES

- 8. THE CONTRACTOR SHALL PROVIDE A QUALITY CONTROL PROGRAM FOR THE CONSTRUCTION REGULATED UNDER 780 CMR 17.00 OF THE 7TH EDITION OF THE MASSACHUSETTS STATE BUILDING CODE, QUALITY ASSURANCE, AND ITS IMPLEMENTATION DOES NOT RELIEVE THE CONTRACTOR OF ITS RESPONSIBILITIES FOR QUALITY CONTROL OF THE CONSTRUCTION, FOR COMPLIANCE WITH THE PROJECT CONSTRUCTION DOCUMENTS, NOR FOR ANY DESIGN FOR WHICH IT IS RESPONSIBLE.
- 9. AS FABRICATION AND CONSTRUCTION PROGRESS, INSPECTION REPORTS AND RECORDS OF TESTS AND INSPECTIONS SHALL BE FORWARDED, BY THE CONTRACTOR, TO THE REGISTERED DESIGN PROFESSIONAL (RDP) FOR REVIEW AND APPROVAL. THE ENGINEER SHALL NOTE ANY UNRESOLVED CONSTRUCTION DEFICIENCIES IN WRITING TO THE BUILDING OFFICIAL AND THE ARCHITECT.

LIGHT GAUGE METAL FRAMING				
ITEM	AGENT	SCOPE	FREQUENCY	
1. LIGHT GAUGE METAL FRAMING QC REVIEW	ENGINEER OF RECORD	-REVIEW OF CONTRACTOR'S FIELD QUALITY CONTROL PROCEDURESREVIEW SCOPE OF TESTING AND INSPECTIONS.		
2. MATERIAL CERTIFICATION	TESTING COMPANY	-REVIEW FOR CONFORMANCE TO CONTRACT DOCUMENTS.		
3. INSTALLATION	TESTING COMPANY	-VERIFY THAT TYPE, SIZE, QUANTITY, LOCATION, DETAILS, AND CONNECTIONS OF FRAMING MEMBERS CONFORM TO RESPONSIBLE RDP APPROVED SUBMITTALS AND THE CONTRACT DOCUMENTS.		
4. WELDING	TESTING COMPANY	-CHECK WELDER QUALIFICATIONSVERIFY THAT WELDING CONFORMS TO AWS SPECIFICATIONS, RESPONSIBLE RDP APPROVED SUBMITTAL, AND THE CONTRACT DOCUMENTSVISUALLY INSPECT WELDS.		
5. OTHER FASTENERS	TESTING COMPANY	-VERIFY FASTENER TYPE AND INSTALLATION PROCEDURESVERIFY THAT FASTENERS CONFORM TO RESPONSIBLE RDP APPROVED SUBMITTALS AND THE CONTRACT DOCUMENTSVERIFY THAT FASTENERS ARE INSTALLED TIGHT.		
6. FIELD CORRECTION OF FABRICATED ITEMS	TESTING COMPANY	-REVIEW DOCUMENTATION OF RESPONSIBLE RDP APPROVED REPAIR AND VERIFY COMPLETION OF REPAIRS.		

MASONRY				
ITEM	AGENT	SCOPE	FREQUENCY	
1. MASONRY CONSTRUCTION QC REVIEW	ENGINEER OF RECORD	- REVIEW CONTRACTOR'S FIELD QUALITY CONTROL PROCEDURES.	PRIOR TO MASONRY CONSTRUCTION	
2. MATERIALS	ENGINEER OF RECORD	- REVIEW MATERIAL CERTIFICATIONS FOR CONFORMANCE TO SPECIFICATIONS.	PRIOR TO MASONRY CONSTRUCTION	
3. EVALUATION OF MASONRY STRENGTH	TESTING COMPANY	- VERIFY STRENGTH IN ACCORDANCE WITH THE SPECIFICATIONS	PRIOR TO MASONRY CONSTRUCTION	
4. PROPORTIONING, MIXING AND CONSISTENCY OF MORTAR AND GROUT	TESTING COMPANY	- INSPECT FIELD-MIXING PROCEDURES FOR CONFORMANCE TO THE SPECIFICATIONS VERIFY PROPORTIONING OF MATERIALS IN MORTAR AND GROUT AS DELIVERED TO THE SITE.	- PRIOR TO CONSTRUCTION	
5. INSTALLATION OF MASONRY	TESTING COMPANY	- INSPECT PLACEMENT FOR CONFORMANCE TO THE SPECIFICATIONS VERIFY CLEANOUT HOLE LOCATIONS (HIGH LIFT GROUTING) - VERIFY THE INSTALLATION OF THE BOND BEAMS AND SPECIAL SHAPES.	- PRIOR TO GROUT PLACEMENT	
6. REINFORCEMENT INSTALLATION	TESTING COMPANY	-INSPECT REINFORCING STEEL FOR SIZE, QUANTITY, CONDITION AND PLACEMENT FOR CONFORMANCE TO RESPONSIBLE RDP APPROVED SUBMITTALS AND CONTRACT DOCUMENTS. - INSPECT WELDING OF REINFORCEMENT AND REVIEW WELDER'S CERTIFICATIONS. - INSPECT MECHANICAL SPLICES.	- PRIOR TO GROUT PLACEMENT	
7. GROUTING OPERATIONS	TESTING COMPANY	 INSPECT GROUTING PROCEDURES FOR CONFORMANCE WITH THE SPECIFICATIONS. INSPECT CELLS PRIOR TO GROUTING. ASSURE OBSERVATION HOLES HAVE BEEN INSTALLED PRIOR TO HIGH LIFT GROUTING. 	PRIOR TO GROUT PLACEMENT	
8. WEATHER PROTECTION	TESTING COMPANY	- INSPECT FOR SIZE, GRADE OF STEEL, CAMBER, INSTALLATION AND CONNECTION DETAILS. - CHECK AGAINST APPROVED CONSTRUCTION DOCUMENTS AND SHOP DRAWINGS. - INSPECT PROTECTION FOR COLD AND HOT WEATHER FOR CONFORMANCE WITH THE SPECIFICATIONS.	ON-GOING AS INSTALLATION PROGRESSES	
9. ANCHORS	TESTING COMPANY	- INSPECT ANCHORAGE OF MASONRY TO OTHER CONSTRUCTION FOR CONFORMANCE TO THE CONTRACT DOCUMENTS.	ON-GOING AS INSTALLATION PROGRESSES	

CAST IN PLACE CONCRETE				
ITEM	AGENT	SCOPE	FREQUENCY	
1. CAST IN PLACE CONCRETE CONSTRUCTION QC REVIEW	TESTING COMPANY	REVIEW CONTRACTOR'S FIELD QUALITY CONTROL PROCEDURES. REVIEW FREQUENCY AND SCOPE OF FIELD TESTING AND INSPECTIONS.	AT START OF PROJECT	
2. MIX DESIGN	TESTING COMPANY	REVIEW MIX DESIGNS PRIOR TO PLACEMENT. VERIFY USE OF REQUIRED MIX DESIGN.	AT START OF PROJECT	
3 MATERIALS	ENGINEER OF RECORD	- REVIEW MATERIAL CERTIFICATIONS FOR CONFORMANCE TO SPECIFICATIONS.	AT START OF PROJECT	
4. BATCHING PLANT	TESTING COMPANY	- REVIEW PLANT QUALITY CONTROL PROCEDURES AND BATCHING AND MIXING METHODS.	AT START OF PROJECT	
5. REINFORCEMENT INSTALLATION	TESTING COMPANY	- INSPECT REINFORCING FOR SIZE, QUANTITY, CONDITION AND PLACEMENT VERIFY SIZE, SPACING, COVER, AND TYING OF DOWELS.	PRIOR TO EACH CONCRETE PLACEMENT	
6. ANCHOR RODS	TESTING COMPANY	- INSPECT ANCHOR RODS FOR SIZE, TYPE, EMBEDMENT AND THAT THEY ARE PROPERLY SECURED PRIOR TO AND DURING PLACEMENT OF CONCRETE.	PRIOR TO CONCRETE PLACEMENT	
7. FORMWORK	TESTING COMPANY	- INSPECT FORM SIZES FOR PROPER SIZES OF CONCRETE MEMBERS.	PRIOR TO EACH CONCRETE PLACEMENT	
8. CONCRETE PLACEMENT AND SAMPLING FRESH CONCRETE	TESTING COMPANY	- OBSERVE CONCRETE PLACEMENT OPERATIONS. VERIFY CONFORMANCE TO SPECIFICATIONS, INCLUDING COLD AND HOT WEATHER PLACEMENT PROCEDURES. - PERFORM SLUMP, DENSITY AND AIR CONTENT TESTS AT POINT OF PLACEMENT.	- DURING EACH CONCRETE PLACEMENT - TEST ONCE FOR EACH BATCH WHERE CYLINDERS ARE TAKEN	
9. EVALUATION OF CONCRETE STRENGTH	TESTING COMPANY	- TEST AND EVALUATE IN ACCORDANCE WITH THE SPECIFICATIONS.	 FOUR CYLINDERS PER BATCH ONCE EACH DAY BUT NOT LESS THA ONCE FOR EACH 150 CUBIC YARDS NOR LESS THAN ONCE FOR EACH 5000 SQUARE FEET OF SLAB AREA 	
10. CURING AND PROTECTION	TESTING COMPANY	- OBSERVE PROCEDURES FOR CONFORMANCE TO THE SPECIFICATIONS.	ON-GOING AS INSTALLATION PROGRESSI	

SPECIAL CASES					
ITEM	AGENT	SCOPE	FREQUENCY		
1. HELICAL PILES	VENDOR'S ENGINEER & GEOTECHNICAL ENGINEER	-OWNER'S GEOTECHNICAL ENGINEER SHALL REVIEW HELICAL PILE DESIGN AND OBSERVE INSTALLATION, THEN ISSUE REPORT OF PILE CAPACITY ATTAINED	ONCE AT PILE DESIGN AND ONCE AT INSTALLATION OF PILES		



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